

Fuerza tn			x(m)	Mv(t*m)	Mh(t*m)	ftensor	Mtensor				y(m)	Mv(t*m)	Mh(t*m)	Mempujes(t*m)
F	V(t)	H(t)												
P1	1.53		0.725	1.10925								1.10925	0.248625	
P2	4.21		1.225	5.15725								5.15725	0.684125	
P3	0.86		0.3	0.258								0.258	0.05805	
P4	1.34		0.8	1.072								1.072	0.02345	
P5	0		1.225	0								0		
tensor			2.90m		0	0.0 Ton	0.00m							
Eeq										0.28	1.93m			0.54124
E		3.08	0.97m		2.97836						0.967			2.97836
E'		0	1.45		0						0			
Σ	7.94	3.08		7.5965	2.97836	0	0		7.94	0.794	3.36	7.5965	1.01425	3.5196
Ep		3.3	0.333		1.0989									

1) Al volcamiento

F.S.=

2.92 OK

F.S.=

2.05 OK

2) Al deslizamiento

F.S.=

2.75 OK

F.S.=

2.15 OK

Revisión del espesor del vástago.

d=

3.67cm

OK

OK

d=	3.67cm
ΣV=	7.94
Mest=	8.70
Mdest=	4.24
x=	0.58m
e=	0.24m

OK

C) Analisis de la presion del suelo:

$\sigma_{adndel\ suelo}$

12.5t/m²

Pto de aplicación de la resultante;

x=

0.72m

Excentricidad:

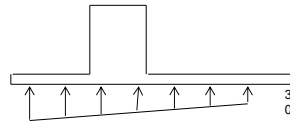
e=B/2-x

0.08m OK la resultante cae en el tercio medio

OK	OK
9.43t/m²	OK
	OK

6.45t/m²

9.43t/m²



3.47t/m²

0.50t/m²

OK	OK
OK	0.50t/m²

d) Diseño estructural del muro.

d1) En el vástago.

h=

0.25m

d=

0.18m

Mu=

3.83t*m

B=

1.00m

3.92

Ea=

3.08t/m²

Eeq=

0.28t/m²

Eae=

3.36t/m²

ρ =

0.00330

As=

5.78cm²

Mu=

0.00t*m

ρ=

0.0033

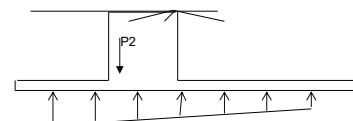
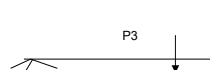
As=

8.25cm²

No	4	c/	0.20m
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No	4	c/	0.15m
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d2) En el apoyo.



ρ MIN=

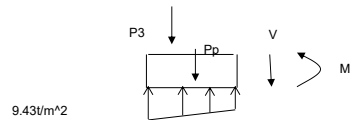
0.0015

As=

3.75cm²

No	4	c/	0.30m
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Para el pie

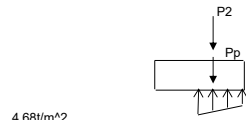


9.43t/m²

B=	1.00m
H=	0.35m
d=	0.29m
Vu=	5.59t OK
Mu=	1.37t*m
ρ =	0.00330
As=	9.57cm²

No	4	c/	0.10m
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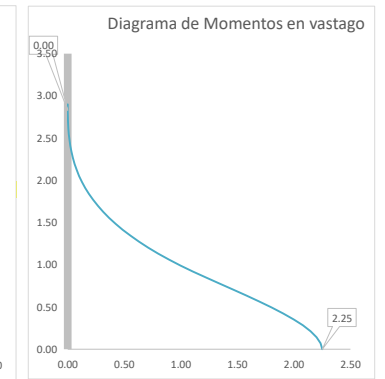
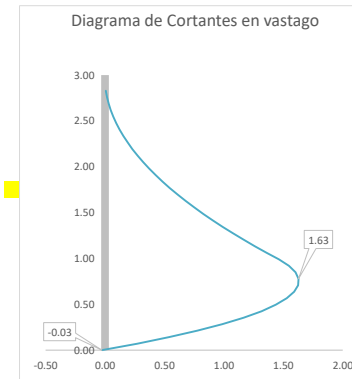
Para el talon



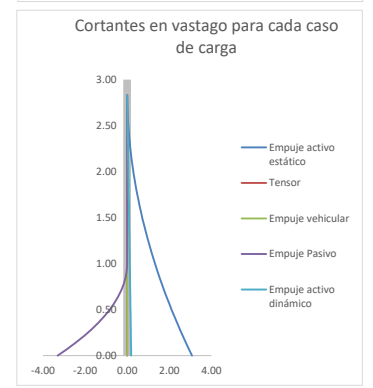
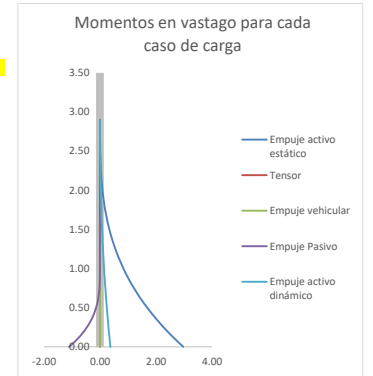
4.68t/m²

B=	1.00m
H=	0.35m
d=	0.29m
Vu=	6.14t OK
Mu=	2.55t*m
ρ =	0.00330
As=	9.57cm²

No	4	c/	0.10m
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Empuje Activo Dinámico
0.07



Configuración geométrica

